

# Reinforcement of Bearing Capacity Foundation Using Single Soil Column Method Fixed Diameter 3.2 Cm With Calcium Carbide Residue (CCR) And Rice Husk Ash (RHA) Mixed Materials

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**Abstract:** Soil reinforcement method is one of the efforts to improve the technical properties of soil such as soil bearing capacity, compressibility and permeability. Soil column method is one of the alternatives to improve physical properties by stabilization to improve soil bearing capacity. This research aims to increase the bearing capacity of the sole foundation by using the soil column method with a mixture of clay, 3% Calcium Carbide Residue (CCR) and 12% Rice Ash Hush (RHA). This research was conducted experimentally in the laboratory using clay test specimens taken from Padamaran Village, OKI, South Sumatra Province, which were put into a test box with dimensions of 1 m x 1 m x 1.4 m. The soil column modeling in this research was carried out using the soil column method. The soil column modeling in this study used a single column variation with a diameter of 3.2 cm with lengths of 40 cm, 46 cm, and 53 cm, respectively. The results obtained from this research are the ultimate soil bearing capacity ( $q_u$ ) of the clay foundation plate before reinforcement and after reinforcement with the soil column method. The bearing capacity of the footprint foundation plate on the largest clay soil occurs in the soil column variation with a length of 40 cm with a diameter of 3.2 cm where the bearing capacity of the clay soil which was originally 140 kPa increased to 21 kPa. In the experimental results of the loading test, the longer the column, the bearing capacity of the column decreases, which may have something to do with the slenderness factor of the column. The slimmer the column, the smaller the compressive strength of the column so that the tendency of the column to bend/collapse becomes greater. This happens because slender columns not only accept axial forces but also take into account the addition of secondary moments due to the slenderness of the column. Then the column cannot withstand the shear load due to the compacted clay soil around the column.

**Keywords:** Soil Column, Bearing Capacity, Rice Ash Hush (RHA), Calcium Carbide Residue (CCR)

## 1. Introduction

Soft clays have special physical and mechanical properties including large pore numbers, high water content, small volume weight and large plasticity index, causing soft clays to have low bearing capacity and large compression. Soft clay soils are not technically suitable for construction of roads, houses and buildings without deep foundations. Problems that may often occur if not using deep foundations are cracks on the surface and even buildings can collapse and on the road there will be a non-uniform decline and even collapse (Iswan, 2017).

Rizolla (2015) analyzed the method of reinforcing the pile foundation compared to the pile foundation on the bearing capacity value of the footprint foundation from the calculation using the Terzaghi analysis method (1943) shows that the footprint foundation using the reinforced pile on load I and load II is 3339.34 kN and 7785.48 kN. Meanwhile, using the Caisson method, the pile foundation obtained the maximum value of the bearing capacity at load I and load II was 3992.82 kN. While using the Caisson method, the pits foundation obtained the maximum value of the bearing capacity at load I and load II was 3992.82kN and reinforcement with the Deep Soil Mixing method with a mixture of clay and 3% carbide waste obtained an

increase in the maximum ultimate bearing capacity ( $q_u$ ) value occurred in the column variation with a length of 53 cm with a diameter of 4.8 cm where the ultimate bearing capacity ( $q_u$ ) value achieved was 11.8 Kpa, the BCR value was 2.242 and the percentage increase in BCR value was 124.2%. The minimum increase in the ultimate bearing capacity ( $q_u$ ) value occurred in the column variation with a length of 53 cm with a diameter of 3.2 cm where the ultimate bearing capacity ( $q_u$ ) value achieved was 7.6 kPa, the BCR value was 1.444, and the percentage increase in BCR value was 44.4% (Astuti, Inda et. al, 2019).

The Soil Column method is one of the alternatives for soil stabilization. The purpose of using Soil Column method is to increase the bearing capacity of clay soil. Calcium Carbide Residue (CCR) was introduced (Vichan et.al., 2013) as a material that can be a substitute for cement because it contains high calcium ions which have the potential as a pozzolanic material when mixed with silica. Carbide waste (CCR) is the remnants of welding that uses carbide gas ( $C_2H_2$ ) as fuel. Carbide waste contains about 60% lime hydroxide ( $Ca(OH)_2$ ). Cementation material can be obtained from carbide waste when mixed with silica ( $SiO_2$ ) because it can form a formation.

( $SiO_2$ ) because it can form pozzolan. Rice Ash Hush

(RHA) is rice husk ash waste that contains high silica elements, the silica content in rice husk ash ranges from 60% - 95%. Both of these materials can be used as a substitute for cement as a binder.

Based on the description above, the authors conducted a study of the bearing capacity of the reinforced footing foundation using the Soil Column method of a single pile with a fixed diameter of 3.2 cm with a mixture of Calcium Carbide Residue (CCR) and Rice Husk Ash (RHA).

## 2. Material and Methods

### 2.1. Materials

The materials used in this research are clay, Rice Husk Ash (RHA) and Calcium Carbide Residue (CCR). The clay was taken from Seriguna Village, Padamaran, Ogan Komering Ilir Regency, South Sumatra. The soil taken is in a state of disturbed soil. Then for RHA (Rice Ash Hush) obtained residue from burning rice husks in Lahat South Sumatra and Calcium Carbide Residue (CCR) waste obtained from welding waste in Cinde Market Palembang. The test box used is made of wood with dimensions with a minimum size of 4 times the width of the foundation (B) which is 60 cm. The test box used measures 1 m x 1 m x 1.4 m.

Soil column used in this study is made of a mixture of clay soil that has been prepared and then baked and filtered to pass sieve No. 04 mixed with rice husk ash waste (RHA) 12% of the weight of the original clay soil and carbide waste (CCR) 3% of the weight of the original clay soil then mixed with the optimum water content of 37.8%. For the total weight of the mixture to be done 2000 grams. Then the total need for RHA is 12% of 2000 grams, namely 240 grams, the need for CCR 3% of 2000 grams, namely 60 grams and the need for clay soil is 2000 grams minus the weight of RHA and CCR so 1700 grams. Then mixed with 37.8% water, namely 756 ml of water and stirred until the mixture was smooth. The soil is put into an impermeable plastic with a curing period of 24 hours to prevent evaporation and the water content is maintained then open the mold in an upright state with full care so that the soil column is not broken as seen in Figure 1.



Figure 1. Columns that have come off the mold

Tabel 1. Variasi Benda Uji

No.	d/L	Diameter (d)	Panjang (L)
1.	0,08	3,2 cm	40 cm
2.	0,07	3,2 cm	46 cm
3	0,06	3,2 cm	53 cm

Variations of single-pile test specimens based on variations in diameter and length of soil column used in this study can be seen in Table 1, then for illustration of different tests can be seen in Figure 2.

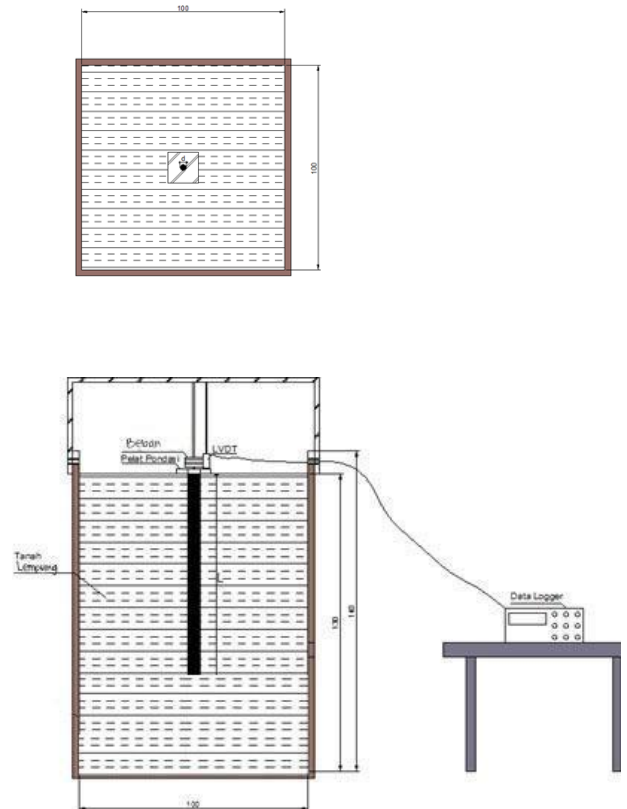


Figure 2. Illustration of the experiment

### 2.2. Methods

#### 2.2.1. Sample collection and preparation

The materials used in this research are clay, Rice Husk Ash (RHA) and Calcium Carbide Residue (CCR). The clay was taken from Seriguna Village, Padamaran, Ogan Komering Ilir Regency, South Sumatra. The soil taken is in a state of disturbed soil. Then for RHA (Rice Ash Hush) obtained residue from burning rice husk in Lahat South Sumatra and Calcium Carbide Residue (CCR) waste obtained from welding waste in Cinde Market Palembang.

#### 2.2.2. Experimental variable and analytical procedures

To determine the bearing capacity of the soil column, a loading test was carried out on the column in a testing box filled with clay soil as high as 1 m and saturated with water for 24 hours. The equipment used in the test included a steel plate load measuring 15 cm x 15 cm x 2 cm, LVDT and data logger. All

instruments must be arranged symmetrically so that the resultant load is parallel to the axis of the test pole (Ferry Fatnanta, A. O. 2018).

The loading test carried out in this study was carried out based on using ASTM D-1143 procedures. The loading procedure carried out in this study is included in the Quick Maintained Load Test (QML) type of loading test. This method is relatively faster than other methods required by ASTM. In the QML test, the load increase is carried out gradually every 5% of the plan load until the load collapse is reached. From the calculation of the plan load using the empirical formula, a load of 141.32 kg was obtained, then 5% of the plan load was 7.066 kg so that for more accurate data an increase in load of 4 kg was used. The increase in load was held for at least 4 minutes but not more than 15 minutes. Recording was done at time periods of 0, 5, 10, 15, and so on at each multiple of 5 minute time interval for each load increase. The loading test in the study was conducted until the pile collapsed. ASTM D 1143 states the test is stopped if the loading reaches 1.5 to 2x the plan load.

### 2.3. Data Analysis

After all testing is done, it is done data analysis of the load and the decrease that occurs which is obtained from the results of the test data. The following will be done in data analysis. Make a data interpretation graph using the P-Y load method for the relationship between settlement and loading to obtain the bearing capacity of the pile and find the value of the pile bearing capacity from empirical calculations. Looking for the BCR (Bearing Capacity Ratio) value in the single column and group column of each test variation.

## 3. Results and Discussion

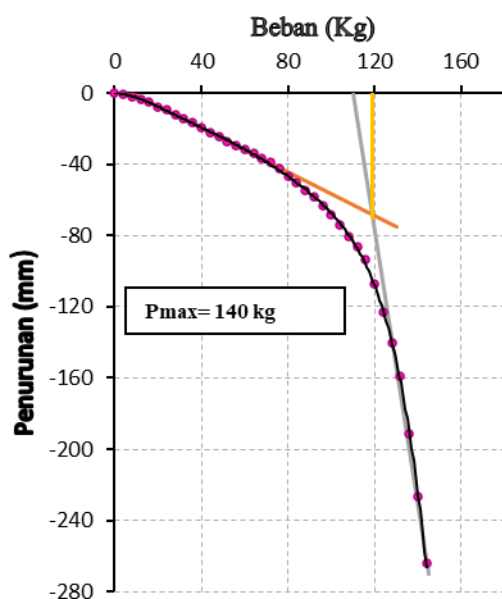


Figure 3. Ultimate load determination graph of foundation plate p-y method.

In laboratory tests for foundation plates without columns, loading tests were carried out in a box by installing a set of loading test equipment and LVDTs, namely frames, data loggers, LVDTs and foundation model plates. From the results of the loading test, the maximum load that can be received by the plate is 140 kg as shown in Figure 3 the calculation shown as:

$$\begin{aligned} q_u &= \frac{P}{A} \\ &= \frac{\text{Beban ultimit plat pondasi}}{A} \\ &= \frac{140 \text{ kg}}{15 \text{ cm} \times 15 \text{ cm}} \\ &= 0,62 \text{ kg/cm}^2 \end{aligned}$$

Using Empirical method to obtain the ultimate bearing capacity value of unreinforced foundation plate using Terzaghi analysis. Soil data required for empirical calculation of bearing capacity are  $C_u$ ,  $\phi$  and  $N_c$  values.

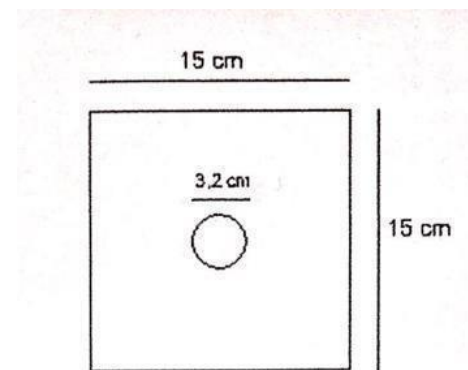


Figure 5. Top view of foundation plate with reinforced single column of 3.2 cm diameter.

Diketahui:

$$C_u = 0,11 \text{ kg/cm}^2 \text{ ( dari pengujian vane shear )}$$

$$\phi = 0^\circ \text{ sehingga nilai } N_c = 5,71$$

Sehingga

$$\begin{aligned} q_u &= C N_c \\ &= (0,11 \text{ kg/cm}^2)(5,71) \\ &= 0,62 \text{ kg/cm}^2 \end{aligned}$$

$$Q_u = q_u \times A$$

$$Q_u = 0,81 \text{ kg/cm}^2 \times (15 \text{ cm} \times 15 \text{ cm})$$

$$Q_u = 141,32 \text{ kg}$$

The test variations carried out amounted to 3 variations. From the results of the loading test, a graph of the relationship between settlement and load was obtained. The ultimate load determination uses the method proposed by Michael T. Adams and James G.

Collins using an interaction diagram, which is two linear lines that intersect the top and bottom of the graph.

Results of loading test of foundation plate with single column variation 1 with soil column size  $d=3.2$  cm,  $L=40$  cm,  $d/L=0.08$ . Top view of foundation plate with single column reinforcement of 3.2 cm diameter Based on Figure 5, the ultimate load of 156 kg was obtained. So that the bearing capacity value of the foundation plate with single column variation 1 is:

$$\begin{aligned} A &= \text{Luas alas pondasi plat} - \text{luas kolom tunggal variasi 1} \\ &= (15 \text{ cm} \times 15 \text{ cm}) - (0,25 \times 3,14 \times 3,2 \text{ cm} \times 3,2 \text{ cm}) \\ &= 225 \text{ cm}^2 - 8,05 \text{ cm}^2 = 216,94 \text{ cm}^2 \end{aligned}$$

Qu plat pondasi dengan kolom tunggal variasi 1

$$\begin{aligned} &= A \times q_u (\text{Plat Pondasi}) \\ &= 216,94 \text{ cm}^2 \times 0,62 \text{ kg/cm}^2 \\ &= 134,99 \text{ kg} \end{aligned}$$

Qu Kolom= 156 kg - 134,99 kg

$$= 21,00 \text{ kg}$$

qu kolom=  $P/A = 21,00 \text{ kg} / 8,04 \text{ cm}^2$

$$= 2,61 \text{ kg/cm}^2$$

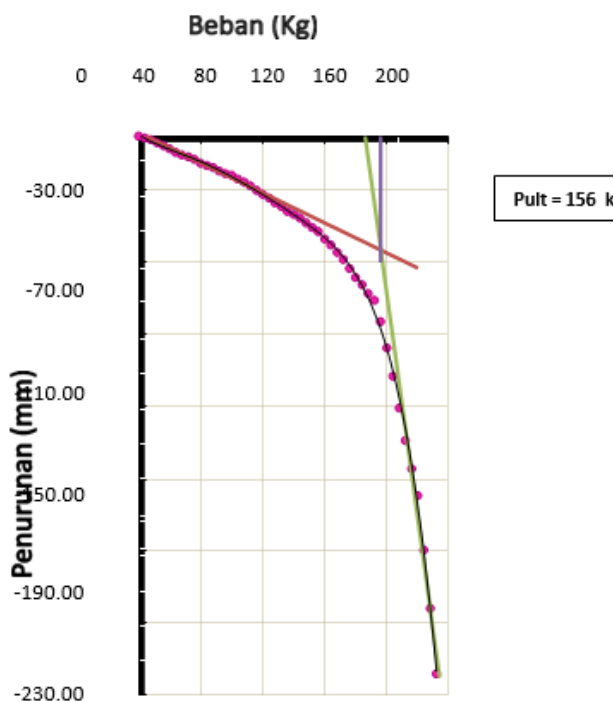


Figure 6. Graph of load and settlement relationship of foundation plate with single column variation 1

Table 2. Recapitulation of single column bearing capacity values

Variasi	Diameter (d)	Panjang (L)	d/L	Pult (kg)	Qult (kPa)
Tanpa Perkuatan (TP)	-	-	-	140	141,322
1	3,2 cm	40 cm	0,08	21	51,499
2	3,2 cm	46 cm	0,07	15	62,497
3	3,2 cm	53 cm	0,06	5	79,247

From the tests that have been carried out on the foundation plate with a single soil column, it shows that there is an increase in the bearing capacity of the foundation plate before after adding a soil column. The increase in bearing capacity value is generated from various variations of soil column with different diameter and length of soil column.

The recapitulation of soil bearing capacity before and after reinforcement for various single column variations can be seen in Table 2. The diagram of the bearing capacity value of a single column with a fixed diameter of 3.2 cm can be seen in Figure 7.

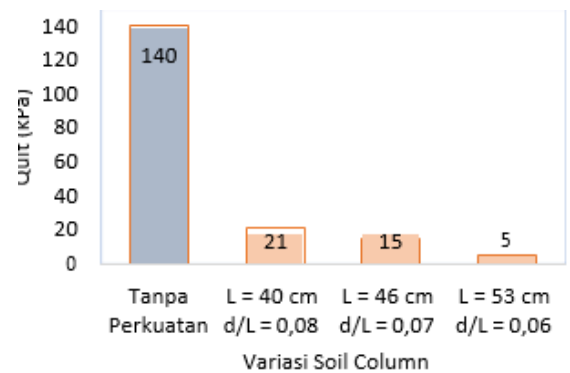


Figure 7. Diagram of bearing capacity value of single column with fixed diameter = 3.2 cm.

In the experimental results of the loading test, the longer the column length, the bearing capacity of the column decreases, which may have something to do with the slenderness factor of the column. The slimmer a column is, the smaller the column compressive strength so that the tendency of the column to bend/collapse becomes greater. This happens because slender columns not only accept axial forces but also take into account the addition of secondary moments due to the slenderness of the column. Then the column cannot withstand the shear load due to the compacted clay soil around the column.

#### 4. Conclusion

Soil column with a mixture of clay, rice husk ash (RHA) 12% and 3% carbide waste (CCR) has an effect in increasing the bearing capacity of the foundation plate. Before soil column reinforcement, the bearing capacity of the foundation plate was 140 kg and there was an increase in bearing capacity after reinforcement with soil column in each single and group column variation. Soil column single pile with a diameter of 3.2 cm obtained the largest bearing capacity value of 21 kg in the soil column

variation at a length of 40 cm. While the smallest pile bearing capacity with a pile bearing capacity value of 5.00 kg in the soil column variation with a length of 53 cm.

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